GEOTECHNICAL EVALUATION GUSTIN LEVEE EVALUATION PARCEL NO. 45344.9108 SPOKANE COUNTY, WASHINGTON

Inland Pacific Engineering Company Project No. 14-037B

July 23, 2015 Revised August 29, 2016





July 23, 2015 Project No. 14-037B

NAI Black c/o Mr. Bryan Walker 107 South Howard Suite 500 Spokane, WA 99201

Re:

Geotechnical Evaluation Gustin Levee Evaluation 4403 South Dishman-Mica Road Spokane County, WA

Dear Mr. Walker:

We have completed the geotechnical evaluation for the Gustin Levee at the above-referenced site in Spokane County, Washington. The purpose of evaluation was to evaluate the existing levee for conformance to 44 CFR 65.10 of the Code of Federal Regulations for certification by the Federal Emergency Management Agency (FEMA).

We appreciate the opportunity to provide our services to you on this project. If you have any questions or need additional information, please do not hesitate to call me at (509) 209-6262 at your convenience.

Sincerely,

Inland Pacific Engineering Company

Paul T. Nelson, P.E. Principal Engineer

Attachment: Geotechnical Evaluation Report

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Prepared for:

NAI Black Spokane, Washington



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1.0 INTRODUCTION

1.1 Project Description

We understand that the proposed Painted Hills project may consist of a residential development. The site consists of 91 acres currently developed as a golf course. Stormwater runoff will be treated using drywells and/or gravel galleries for subsurface infiltration. These type of facilities will also be used to manage potential floodwaters, if needed. In addition, runoff from the drainage east of the site will require a FEMA certified levee. The existing levee is located on the Gustin property off State Highway 27.

1.2 Purpose

The purpose of our services is to evaluate the existing levee for conformance to 44 CFR 65.10 of the Code of Federal Regulations for certification by the Federal Emergency Management Agency (FEMA).

1.3 Scope

Our services were requested by Mr. Bryan Walker of NAI Black. Mr. Walker authorized us to proceed on April 28, 2015. The scope of work agreed upon consisted of the following:

- review of existing geotechnical data and reports for the development, if available
- drill 8 penetration test borings at the site to a depth of 15 feet,
- excavate 6 test pits through the existing levee embankment,
- performing laboratory tests on samples obtained from the test pits,
- classifying the soils and preparing boring logs, and
- submitting a geotechnical report containing logs of the borings, results of our field investigation and laboratory testing, and our analyses, opinions, and recommendations relative to the conformance of the existing levee to FEMA standards.

1.4 Available Information

We were provided a topographic survey for the project site by WCE. This topographic survey showed the existing roadways, existing structures, property lines, and existing ground surface elevation contours. This plan was prepared by WCE and was not dated.

We also performed a preliminary geotechnical evaluation for the golf course property in December 2013. The results of that evaluation, along with our opinions and recommendations, are summarized in our Preliminary Geotechnical Evaluation dated December 31, 2013.

In conjunction with this evaluation, West Consultants, Inc. (WEST) has been contracted by NAI Black to provide a FEMA Conditional Letter of Map Revision submittal (CLOMR). They have provided Inland Pacific Engineering Company (IPEC) water surface elevations and velocity output from their revised RAS model to assist us in our evaluation.

1.5 Locations and Elevations

The borings and test pits were drilled and/or excavated at or near locations selected by us. The boring and test pit locations are shown on the Boring Location Map in Appendix A. The borings

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were staked by Whipple Consulting Engineers, Inc. (WCE). The test pits were staked by IPEC. Ground surface elevations at the borings were provided by WCE. Ground surface elevations at the test pits were interpolated from the topographic survey provided by WCE.

2.0 RESULTS

2.1 Logs

Log of Boring and Log of Test Pit sheets indicating the vertical sequence of soils and materials encountered and groundwater observations are included in Appendix B. The strata changes were inferred from the changes in the penetration test samples and auger cuttings brought to the surface or measured from the surface in the test pits. Please note that the depths shown as changes between the strata are only approximate. The changes are likely transitions and the depths of changes vary between the borings. Geologic origins for each stratum are based on the soil type, available geologic maps, previous geotechnical reports for this and adjacent sites, and available common knowledge of the depositional history of the site.

2.2 Site Conditions

The site is an open field with no trees and is primarily grass-covered. The existing levee extends from the culvert under State Highway 27 on the east side of the property and extends west to the west side of the parcel. The creek side of the levee is typically at a 1.5:1 to 1:1 (H:V) slope. The land side of the levee is typically at a 2.5:1 to 1.5:1 slope. There are two existing crossings where the levee embankment is not present. The crossings consist of embankments with culverts.

2.3 Soils

Geologic maps indicate the soils in this area consist primarily of alluvial and/or glacially deposited silts, clays, sands, and gravels. According to the Soil Survey of Spokane County, the site soils are classified by the Natural Resource Conservation Service (NRCS) as Hardesty ashy silt loam and Urban land-Springdale, disturbed complex. The native soils encountered in the borings and test pits were consistent with the NRCS data.

The borings encountered approximately 1 foot of topsoil at the surface. Below the topsoil, the borings encountered alluvial or glacial silty to clayey sands and/or poorly graded sands to their termination depths. Borings B-1 and B-3 encountered silty clay between the 4 and 9-foot depths.

The test pits were excavated through the existing levee embankment. They encountered existing fill in the upper 4 to 5½ feet. The embankment fill consisted primarily of silty sand. Below the fill, the test pits encountered alluvial or glacial silty to clayey sands and/or gravels to their termination depths.

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2.4 Penetration Resistances

Penetration resistances (N-values) in the sands and gravels ranged from 6 to 59 blows per foot (BPF) and averaged 23 BPF indicating that these soils were loose to very dense, but were typically medium dense. Penetration resistances in the clays ranged from 8 to 18 BPF indicating they were medium to very stiff in consistency.

2.5 Groundwater

Groundwater was not encountered in the borings or test pits. Groundwater is believed to currently exist at some depth below the termination depths of the borings and test pits. Well log data in the vicinity of the site indicate that groundwater is typically 50 to 80 feet below the surface.

2.6 Laboratory Testing

We obtained soil samples from the borings during our site investigation. The tests performed included the following:

- 1. ASTM D 6913, Sieve Analysis
- 2. ASTM D 4318, Atterberg Limits'

These tests were used to aid in classifying the soils and in the engineering analyses and formulation of engineering opinions and recommendations. The tests were performed by IPEC. Attached are data sheets summarizing the tests performed.

3.0 ANALYSIS AND RECOMMENDATIONS

3.1 Discussion

Based on the results of the borings and test pits, along with topographic data from WCE, the existing levee will need significant modification in order to meet the FEMA standard. At this time, we understand that the existing crossings will be removed. As such, additional freeboard will not be required at these locations. The sideslopes of the levee will need to be sloped flatter to maintain slope stability during flooding. In addition, the crown will need to be widened. It is our opinion that the levee embankment materials are suitable but will need to be re-worked to provide adequate flood protection. In summary, the levee as it exists cannot be certified at this time.

3.2 History

The levee was constructed prior to 1945, when the Gustin family purchased the land. As such, design plans or as-built drawings are not available. However, an as-built survey was completed by WCE in 2015.

3.3 Freeboard

We were provided 100-year flood elevations by WEST. They provided us a plan view of the levee with flood elevations at 9 locations starting at the culvert on State Highway 27 and ending

at the proposed triangle stormwater pond north of 40th Avenue west of the Gustin property. The elevations ranged from 2020.02 at State Highway 27 to 2008.85 at the triangle pond. Please refer to the WEST report for a complete summary of the floodplain analysis.

According to 44 CFR Section 65.10(b)(1), an additional 1 foot of freeboard is required within 100 feet of bridge structures. This will require the top of the levee to be at elevation 2024.02 at the State Highway 27 crossing and extending 100 feet west. Since the existing levee is at approximately elevation 2020 in this area, it will need additional fill to meet the minimum freeboard requirements.

3.4 Closures

There are no penetrations of the levee so closure devices are not required.

3.5 Embankment Protection

The levee is currently grass-covered for erosion protection. We evaluated the erosion protection for the creek side of the levee using the results of the HEC-RAS analysis by WEST. They provided flow velocities for the 100-year flood event. The flow velocities are shown in the following table.

FEMA FIS Station	RAS Station	100-year Channel Velocity (ft/sec)
1962.72	586	3.71
2027.72	651	2.67
2295.08	918	1.26
2848.82	1472	4.34
2886.75	1510	2.15
2904.75	1528	0.91
2933.32	1557	1.33
3338.84	1963	2.30
3364.84	1989	2.11

As shown in the table, the average flow velocity is typically less than 3 feet per second. At these velocities, it is our opinion that the vegetative erosion protection is adequate for levee slope protection.

3.6 Embankment and Foundation Stability

We evaluated the embankment and foundation stability for conditions described in EM 1110-2-1913, "Design and Construction of Levees, by the US Corps of Engineers dated April 30, 2000, Chapter 6. We analyzed the existing levee embankment for the following cases:

- 1. CASE I. End of construction.
- 2. CASE II: Sudden drawdown.
- 3. CASE III: Steady state seepage from full flood stage.

We performed slope stability analyses for each case. We analyzed the levee embankment with 1:1 slopes. This configuration is considered to have the lowest factor of safety. For our analyses, we used XSTABL software which is based on a software program developed at Purdue University.

For these cases, we calculated the minimum factors of safety as shown in the following table.

CASE	Minimum Factor of Safety
I	1.03
II	0.92
III	0.50

For stability, a minimum factor of safety of 1.5 is generally considered acceptable. Based on this analysis, it is our opinion that the levee will not be stable with respect to global slope stability with its current geometry.

We recommend that the levee be improved to have maximum 3:1 (H:V) slopes for stability. We recommend a minimum crown width of 8 feet. We recommend that the levee embankment materials consist of a granular soil having 10 to 30 percent by weight passing a 200 sieve to reduce the permeability and limit seepage. We have assumed that on-site soils will be used as borrow to construct the embankment.

3.7 Settlement

The average depth of fill is approximately 5 feet. This would result in a loading increase of approximately 625 pounds per square foot (psf) on the bearing soils. Based on the data obtained from the borings, the levee was constructed above medium dense sands. Settlement in these soils would have occurred shortly after construction. Also, given the age of the levee, it is our opinion that significant additional long term settlement will not occur even with addition of 1 to 2 feet in areas requiring additional fill to meet the freeboard requirements.

3.8 Interior Drainage

Interior drainage systems have been designed by WCE. We understand that these systems will include the proposed triangle stormwater pond with multiple drywells to control flood waters and infiltrate them into the ground. Please refer to the WCE report for a comprehensive description of the interior drainage system.

3.9 Operation Plans

The Operation Plan will be prepared as part of the final levee certification.

3.10 Maintenance Plan

The Maintenance Plan will be prepared as part of the final levee certification.

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4.0 PROCEDURES

4.1 Excavation and Sampling

The borings were completed on July 13 and 14, 2015 using a truck-mounted drill rig operated by an independent firm working under subcontract to IPEC. After we logged the borings, they were abandoned in accordance with state requirements. The test pits were excavated on July 21, 2015 using a rubber-tired backhoe operated by an independent firm working under subcontract to IPEC. After we logged the test pits, the test pits were backfilled. A geotechnical engineer from our firm continuously observed the borings and test pits and logged the surface and subsurface conditions.

4.2 Soil Classification

The soils encountered in the borings were visually and manually classified in the field by our field personnel in accordance with ASTM D 2488, "Description and Identification of Soils (Visual-Manual Procedures)".

5.0 GENERAL RECOMMENDATIONS

5.1 Basis of Recommendations

The analyses and recommendations submitted in this report are based on the data obtained from the borings and test pits performed at the locations indicated on the Boring and Test Pit Location Map in Appendix A. It should be recognized that the explorations performed for this evaluation reveal subsurface conditions only at discreet locations across the project site and that actual conditions in other areas could vary. Furthermore, the nature and extent of any such variations would not become evident until additional explorations are performed or until construction activities have begun. If significant variations are observed at that time, we may need to modify our conclusions and recommendations contained in this report to reflect the actual site conditions.

5.2 Groundwater Fluctuations

We made water level observations in the borings and test pits at the times and conditions stated on the boring and test pit logs. These data were interpreted in the text of this report. The period of observation was relatively short and fluctuation in the groundwater level may occur due to rainfall, flooding, irrigation, spring thaw and other seasonal and annual factors not evident at the time the observations were made. Design drawings and specifications and construction planning should recognize the possibility of fluctuations.

5.3 Use of Report

This report is for the exclusive use of the addressee and the copied parties to use in design of the proposed project and to prepare construction documents. In the absence of our written approval, we make no representations and assume no responsibility to other parties regarding this report. The data, analyses, and recommendations may not be appropriate for other structures or purposes. We recommend that parties contemplating other structures or purposes contact us.

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5.4 Level of Care

Services performed by the geotechnical engineers for this project have been conducted in a manner consistent with that level of care ordinarily exercised by members of the profession currently practicing in this area under similar budget and time restraints. No warranty, expressed or implied, is intended or made.

5.5 Professional Certification

This report was prepared by me or under my direct supervision and I am a duly registered engineer under the laws of the State of Washington.

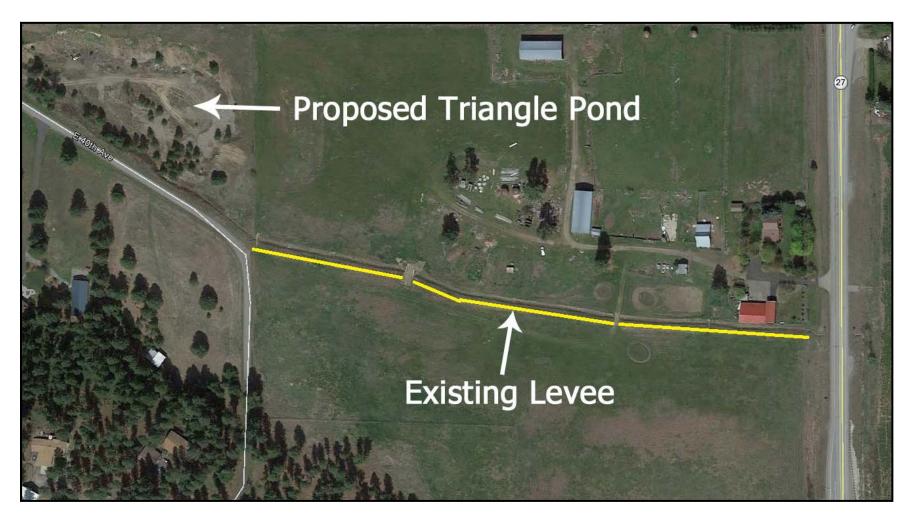
Paul T. Nelson, P.E. Principal Engineer



APPENDIX A

SITE LOCATION MAP, NRCS MAP, BORING AND TEST PIT LOCATION MAP

FIGURE 1



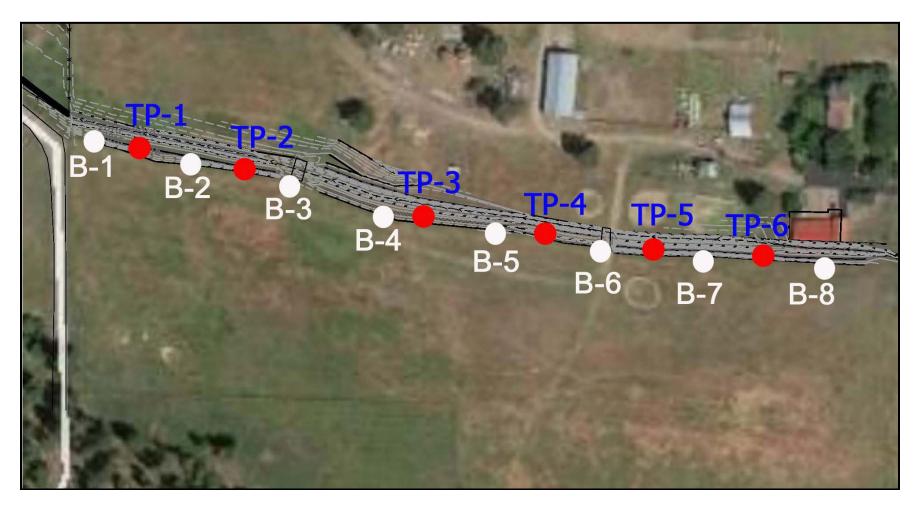
Site Location Map										
IDEC	Project No. 14-037B									
IPEC	Gustin Levee	July 22, 2015								
Inland Pacific Engineering Company	Parcel No. 45344.9108									
Geotechnical Engineering and Consulting	Spokane County, WA									

FIGURE 2



NRCS Map										
IDEC	Project No. 14-037B									
IPEC	Gustin Levee	July 22, 2015								
Inland Pacific Engineering Company	Parcel No. 45344.9108	0419 22, 2015								
Geotechnical Engineering and Consulting	Spokane County, WA									

FIGURE 3



Boring Location Map										
IDEC	Project No. 14-037B									
IPEC	Gustin Levee	July 22, 2015								
Inland Pacific Engineering Company	Parcel No. 45344.9108									
Geotechnical Engineering and Consulting	Spokane County, WA									

APPENDIX B

LOGS OF BORINGS, LOGS OF TEST PITS, DECRIPTIVE TERMINOLOGY



BORING NUMBER B-1

CLIEN	IT <u>N</u> /	N Black	PROJECT NAME Gustin Levee										
PROJ	ECT N	UMBER _14-037B	PROJECT LOCATION Spokane County										
DATE	STAR	TED <u>7/13/15</u> COMPLETED <u>7/13/15</u>	GROUN	D ELEVA	TION _	2010.8 ft		HOLE	SIZE	8 inc	hes		
DRILL	ING C	ONTRACTOR Johnson Exploration Drilling	GROUN	D WATER	LEVE	LS:							
DRILL	ING M	ETHOD Hollow Stem Auger	Α	TIME OF	DRIL	LING N	lot end	counte	red				
LOGG	ED BY	CHECKED BY PTN	A	END OF	DRILL	.ING N	ot enc	ountei	ed				
				1				1	1	AT	ΓERBE	RG	 -
	0			SAMPLE TYPE NUMBER	% /	(0 (ii)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)		LIMITS	}	FINES CONTENT (%)
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DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION		1PLI	SS.	BLOW COUNTS (N VALUE)	쑮ᇵ	58	ISE ESE	LIQUID	ST MT	E)	ပ်စ
	اق			SAN	RECOVERY (RQD)	02	PO	R	ŽŌ	= =	PLASTIC LIMIT	PLASTICITY INDEX	Ä
0	7. N. V.	(CM) CILTY CAND, fine grained with roots, dark brown m	oiot	-								Δ.	ഥ
	17.31,	(SM) SILTY SAND, fine grained, with roots, dark brown, m (Topsoil)	oist.										
-		(SM) SILTY SAND, fine to medium grained, brown, moist,	loose.	-									
5		(Alluvium)											
<u>-</u>				Mag	1	5-4	1						
				X ss		(9)							
		(0) 111 211 211 211 211 211 211 211 211 21											
		(CL-ML) SANDY SILTY CLAY, brown, moist to wet, mediu stiff.	m to		-		-						
5		(Alluvium)		X ss		4-4 (8)			21	24	20	4	77
					-	(0)	-						
<u>-</u>													
-					1	6-8	1						
				X SS		(14)							
							1						
[
		(SM) SILTY SAND, fine to medium grained, a trace of Gra- brown, moist, medium dense.	vel,		-		-						
10		(Alluvium)		ss		12-13 (25)							
					-	(23)	-						
<u>-</u>													
= -		(SM) SILTY SAND with GRAVEL, medium to coarse grains	ed,	1									
		brown, moist to wet, medium dense. (Glacial Outwash)											
		(
2					-		1						
15				ss		14-15 (29)							
	hallaki			VV		(20)							
		End of boring.											
		Groundwater not encountered with 14' of hollow-stem auge	r in the										
[ground.											
		Groundwater not encountered immediately after withdrawa	l of the										
		auger.	•										
		Bore hole then grouted to the surface.											



BORING NUMBER B-2 PAGE 1 OF 1

CLIENT NAI Black													
PROJ	ECT N	UMBER <u>14-037B</u> F	PROJECT LOCATION Spokane County										
DATE	STAR	TED <u>7/13/15</u> COMPLETED <u>7/13/15</u> C	GROUND ELEVATION 2010.7 ft HOLE SIZE 8 inches										
DRILL	ING C	ONTRACTOR Johnson Exploration Drilling C	_ GROUND WATER LEVELS:										
DRILL	ING M	ETHOD Hollow Stem Auger											
LOGG	ED BY	<u>DD</u> CHECKED BY PTN											
NOTE	s												
				111			l .			ATT	ERBE	RG	F
_	ಲ			SAMPLE TYPE NUMBER	%	, s (POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	<u> </u>	_IMITS		FINES CONTENT (%)
DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION		LE T MBE	RECOVERY (RQD)	BLOW COUNTS (N VALUE)	ET (st)	SG I	ISE.	∟∟	PLASTIC LIMIT	SI	Š.
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, 0				S	R		<u>A</u>	造	-8	-	룝	PLASTICITY INDEX	Ξ
<u> </u>	1. N. 1/2	(SM) SILTY SAND, fine grained, with roots, dark brown, mo	ist.										
	11 111	(Topsoil)											
		(SM) SILTY SAND, fine to medium grained, brown, moist, lo medium dense.	ose to										
		(Alluvium)		\ /			-						
				X ss		3-3 (6)							
-				/ <u> </u>			1						
9				\									
5				X ss		7-8 (15)							
5				/ \		(,	1						
j													
5				\bigvee ss		6-8							34
				/\		(14)	-						
Š													
2 -													
10				V ss	1	5-8	1						
5				$\sqrt{}$		(13)							
<u>-</u>													
3													
-		(SP) POORLY GRADED SAND with GRAVEL, medium to c	oarse										
<u> </u> -		grained, brown, moist, medium dense. (Glacial Outwash)											
		•											
4.5				\ /	} }	15 11	1						
15				X ss		15-11 (26)							6
				· •				•	!		•		
3		End of boring.											
		Groundwater not encountered with 14' of hollow-stem auger ground.	in the										
		Groundwater not encountered immediately after withdrawal auger.	of the										
		Bore hole then grouted to the surface.											



BORING NUMBER B-3

CLIEN	NT NA	Al Black PR	PROJECT NAME Gustin Levee									
PROJ	ECT N		PROJECT LOCATION Spokane County									
DATE	STAR	TED _7/13/15	OUND ELEVA	TION _	2012.4 ft		HOLE	SIZE	8 inc	hes		
DRILI	ING C	ONTRACTOR Johnson Exploration Drilling GR	OUND WATER	R LEVE	LS:							
		ETHOD Hollow Stem Auger	AT TIME O									
		CHECKED BY PTN										
NOTE	:S			LLING	Not er	Count	erea	1		ERBE	DC.	
O DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	LIQUID	LIMITS		FINES CONTENT (%)
i	1, 1, 1,	(SM) SILTY SAND, fine grained, with roots, dark brown, moist (Topsoil)	:-									
		(SM) SILTY SAND, very fine to fine grained, brown, moist, loo (Alluvium)	se.									
			ss		3-3 (6)							
		(CL-ML) SANDY SILTY CLAY, brown, moist to wet, very stiff. (Alluvium)				-						
5			X ss		8-10 (18)	_		17	23	19	4	62
		(SP-SM) POORLY GRADED SAND with SILT, medium to coa grained, a trace of Gravel, brown, moist, medium dense. (Glacial Outwash)	ss SS		7-6 (13)							
10_			ss		7-9 (16)							8
15			SS		9-9 (18)							
		End of boring.										
		Groundwater not encountered with 14' of hollow-stem auger in ground.	the									
		Groundwater not encountered immediately after withdrawal of auger.	the									
		Bore hole then grouted to the surface.										



BORING NUMBER B-4

CLIEN	NT NA	Al Black	PROJECT NAME Gustin Levee										
PROJ	ECT N	UMBER 14-037B											
DATE	STAR	TED <u>7/13/15</u> COMPLETED <u>7/13/15</u>	GROUND ELEVATION _2013 ft HOLE SIZE _8 inches										
DRILL	ING C	ONTRACTOR Johnson Exploration Drilling											
DRILL	ING M	ETHOD Hollow Stem Auger											
LOGG	SED BY	<u>DD</u> CHECKED BY PTN											
NOTE	s												
				111	. 0		l .			ATT	ERBE	RG	F
_	ပ			SAMPLE TYPE NUMBER	%	, s (POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	l	IMITS	<u>} </u>	FINES CONTENT (%)
DEPTH (ff)	GRAPHIC LOG	MATERIAL DESCRIPTION		HE	RECOVERY (RQD)	BLOW COUNTS (N VALUE)	ET F	Sch Z	ISE.	∟∟	PLASTIC LIMIT	은 ^X	NO ®
H	GR/			MA	S R	₫ Ö >	S S	⊃ S ≿	NS T	8≅	-AS	ES E) SE
, 0				δ	품		M	造	28		П_	PLASTICITY INDEX	Z Z
<u> </u>	71 18	(SM) SILTY SAND, fine grained, with roots, dark brown, m	oist.										
-	11.711	(Topsoil)											
		(SM) SILTY SAND, fine to medium grained, brown, moist, (Alluvium)	loose.										
		`		\	-	4.4	1						
3				X ss		4-4 (8)							36
-				V V	1		1						
) 		(SP) POORLY GRADED SAND with GRAVEL, medium to grained, brown, moist, medium dense to dense.	coarse	1	-		-						
5		(Glacial Outwash)		X ss		10-9 (19)							
5				/ V	1	(10)	1						
<u>-</u>													
3				ss		6-7							
				/ \	-	(13)	-						
Š													
2 -													
10				V ss	1	20-21	1						
5				\triangle		(41)							
<u>-</u>													
3													
-													
45					1	40.40	1						
15				X ss		13-13 (26)							4
Š				r 4									
3		End of boring.											
		Groundwater not encountered with 14' of hollow-stem auge ground.	er in the										
		Groundwater not encountered immediately after withdrawa auger.	I of the										
		Bore hole then grouted to the surface.											
<u> </u>													



BORING NUMBER B-5

CLIENT	NAI Black	PROJECT NAME	Gusti	n Levee							
	NUMBER 14-037B				County						
1		GROUND ELEVATION 2013.9 ft HOLE SIZE 8 inches									
	CONTRACTOR Johnson Exploration Drilling										
DRILLING	METHOD Hollow Stem Auger	AT TIME OF DRILLING Not encountered									
LOGGED	BY SLN CHECKED BY PTN										
NOTES _		AFTER DR	ILLING	Not er	ncount	ered					
O DEPTH (ft) GRAPHIC	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	LIMIT LIMIT	PLASTIC LIMIT		FINES CONTENT (%)
7/1/4	(SM) SILTY SAND, fine to mediium grained, with roots, of brown, moist. (Topsoil) (SM) SILTY SAND with GRAVEL, fine to medium grained moist, medium dense. (Glacial Outwash) (SP-SM) POORLY GRADED SAND with SILT and GRAV medium to coarse grained, brown, moist, medium dense. (Glacial Outwash)	ss SEL,		11-10 (21) 15-10 (25) 5-6 (11) 6-9 (15)							8
	End of boring. Groundwater not encountered with 14' of hollow-stem auground. Groundwater not encountered immediately after withdraw auger. Bore hole then grouted to the surface.			. ,							



BORING NUMBER B-6

	CLIEN	IT NA	Il Black	PROJEC	T NAME	Gusti	n Levee							
	PROJ	PROJECT NUMBER 14-037B PROJECT LOCATION Spokane County DATE STARTED 7/14/15 COMPLETED 7/14/15 GROUND ELEVATION 2015.1 ft HOLE SIZE 8 inches												
				GROUNE	ELEVA	TION _	2015.1 ft		HOLE	SIZE	8 inc	hes		
	DRILL	ING C	ONTRACTOR Johnson Exploration Drilling											
	DRILL	ING M	ETHOD Hollow Stem Auger	AT TIME OF DRILLING Not encountered										
			SLN CHECKED BY PTN	AT END OF DRILLING Not encountered										
	NOTE	s		AF	TER DRI	LLING	Not er	ncount	ered					
					Ж	%		ż	<u>-</u>	@	ATT 	ERBE	ERG S	LNI
71	o DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	RECOVERY (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	LIQUID		PLASTICITY INDEX	FINES CONTENT (%)
2014 PROJECTS/14-037B GUSTIN LEVEE FOR PAINTED HILLS\GINT/14-037B GUSTIN LEVEE.GPJ		1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1	(SM) SILTY SAND, fine to mediium grained, with roots, darl brown, moist. (Topsoil)	k										
STIN			(SM) SILTY SAND with GRAVEL, fine to coarse grained, br moist, dense to medium dense.	own,										
7B GU			(Glacial Outwash)		\		16-16	-						
14-03					X ss		(32)							
GINT					,									
HLLS)														
TED !	5				Maa		7-15	1						
PAIN					X ss		(22)							
E FOF			(SP) POORLY GRADED SAND with GRAVEL, medium to o	noareo.										
LEVE			grained, brown, moist, very dense to medium dense.	Juaise										
NITS			(Glacial Outwash)		M ss	-	43-15	1						
7B GU					\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \		(58)							
14-03														
ECTS/														
ROJE	10				√ ss		17-16							
2014					\bigwedge		(33)							
- 11														
ROJEC														
EC P	_													
÷.														
5:33 -														
3/15 1														
3DT - 7/2	15				ss		12-12 (24)							
US LAB.			End of boring.											
INT STD			Groundwater not encountered with 14' of hollow-stem auger ground.	in the										
10G - G			Groundwater not encountered immediately after withdrawal auger.	of the										
IPEC BORING LOG - GINT STD US LAB.GDT - 7/23/15 15:33 - J._ IPEC PROJECTS\			Bore hole then grouted to the surface.											
IPEC														



BORING NUMBER B-7

CLIE	NT NA	I Black PF	PROJECT NAME Gustin Levee										
PROJ	IECT N	UMBER 14-037B PF	ROJECT	LOCAT	TION _	Spokane C	County						
DATE	STAR	FED 7/14/15 COMPLETED 7/14/15 GF	ROUND	ELEVA	TION _	2015.9 ft		HOLE	SIZE	8 inc	hes		
DRILI	LING C	ONTRACTOR Johnson Exploration Drilling GF	ROUND	WATER	LEVE	LS:							
DRILI	LING M	ETHOD Hollow Stem Auger	AT	TIME OF	DRIL	LING N	Not end	counte	red				
LOGG	SED BY	SLN CHECKED BY PTN	AT	END OF	DRILL	.ING N	ot enc	ounte	red				
NOTE	S		AFTER DRILLING Not encountered										
				111			Ι.			ATT	ERBE	RG	L
_	ပ			SAMPLE TYPE NUMBER	% ∖.	s (i	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	L	_IMITS	<u>`</u>	FINES CONTENT (%)
DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION		## ##	RECOVERY (RQD)	BLOW COUNTS (N VALUE)	ET F	Sch T		∟∟	PLASTIC LIMIT	PLASTICITY INDEX	NO.
	GR/			MPI	CO (R	B COI	S,	⊃ & ≿	S T	LIQUID	-AS-	STI)
, 0				SA	RE)	N	占	28		립_	PLA =	E I
	1/2 1/2	(SM) SILTY SAND, fine to mediium grained, with roots, dark brown, moist.											
-		(Topsoil) (SM) SILTY SAND with GRAVEL, fine to coarse grained, brown	wn.										
		moist, medium dense. (Glacial Outwash)	ĺ										
8		(Clacial Cutwasii)		$\sqrt{ \mathbf{s} }$		5-13							
-		(SP) POORLY GRADED SAND with GRAVEL, medium to co	arse	<u> </u>	-	(18)	-						
5		grained, brown, moist, dense to very dense.	Jaroc										
-		(Glacial Outwash)											
55				√ ss		24-18							
			4	$\sqrt{}$		(42)							
[- -													
-				V	1	25-16							
				X ss		(41)							
-													
10					-	8-15	1						
- 10				X ss		(23)							
			ľ	,			1						
<u>-</u>													
:													
2													
15	-			X ss		50							
	properties.		<u>V</u>	Ų					1		ı		
		End of boring.											
		Groundwater not encountered with 14' of hollow-stem auger in ground.	in the										
		Groundwater not encountered immediately after withdrawal or auger.	of the										
		Bore hole then grouted to the surface.											
4													



BORING NUMBER B-8

	CLIEN	NT NA	l Black	PROJECT NAME Gustin Levee										
	PROJ	ECT N												
			FED 7/14/15 COMPLETED 7/14/15	GROUNE	ELEVA1	TION _	2020 ft		HOLE	SIZE	8 inc	hes		
	DRILL	ING C	ONTRACTOR _Johnson Exploration Drilling	GROUNE	WATER	LEVE	LS:							
	DRILL	ING M	ETHOD Hollow Stem Auger											
	LOGG	SED BY	SLN CHECKED BY PTN											
	NOTE	:s		AFTER DRILLING Not encountered										
Ī					Й	%		j	<u>.</u>	(9)	ATT	ERBE	RG	L
رد	o DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	RECOVERY (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	LIQUID		PLASTICITY INDEX	FINES CONTENT (%)
LEVEE.GI		1/ 1/1/ 1/ 1/1/	(SM) SILTY SAND, fine to mediium grained, with roots, dar brown, moist. (Topsoil)	⁺k										
JSTIN			(SM) SILTY SAND with GRAVEL, fine to coarse grained, by moist, medium dense.	rown,										
4-037B GI			(Glacial Outwash)		ss		9-8 (17)	_						
S\GINT\1					V N		,	_						
TED HILI	 5				M ss		5-7	_						
FOR PAIN					X ss		(12)	-						
IN LEVEE														
7B GUST			(CD CM) DOODLY CDADED CDAVEL with CU T and CAN	ID fine	ss		7-7 (14)							
2014 PROJECTS/14-037B GUSTIN LEVEE FOR PAINTED HILLS/GINT/14-037B GUSTIN LEVEE.GPJ			(GP-GM) POORLY GRADED GRAVEL with SILT and SAN grained, brown, moist, dense to very dense. (Glacial Outwash)	iD, line										
4 PROJE	10				ss		20-19 (39)	-						7
- 11					/ \ 		,	-						
PROJEC														
- J:_IPE(
1,15 15:33														
GDT - 7/23	15				ss		27-32 (59)							
O US LAB.			End of boring.											
GINT ST			Groundwater not encountered with 14' of hollow-stem auge ground.	r in the										
NG LOG -			Groundwater not encountered immediately after withdrawal auger.	l of the										
IPEC BORING LOG - GINT STD US LAB.GDT - 7/23/15 15:33 - J._IPEC PROJECTS\			Bore hole then grouted to the surface.											



TEST PIT NUMBER TP-1

		Fax:	509-	290-5734					
	T NALE					PROJECT NAME Gust			
1						PROJECT LOCATION _		-	
								TEST PIT SIZE 30 inc	hes
						GROUND WATER LEVE			
		METHOD Backhoo						- Not encountered	
				CHECKED	BY PTN				
NOTES	<u> </u>					AFTER EXCAVAT	TION Not	encountered	
O DEPTH O (ft)	SAMPLE TYPE NUMBER	TESTS	U.S.C.S.	GRAPHIC LOG		MATERIAL [
2.5		MC = 16% Fines = 44%	SM	5.0	(SM) FILL: Silt	y Clayey Sand, fine	grained, a	trace of fibers, brown,	
5.0			SC- SM	5.0	(SC-SM) SILTY (Alluvium)	CLAYEY SAND, fir	ne grained	, brown, moist.	2011.0
				7.0					2009.0
					End of test pit. Groundwater notest pit immedi	ot encountered iately backfilled.			
5									

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TEST PIT NUMBER TP-2 PAGE 1 OF 1

CLIENT NAI Blac	Fax:	phone: 509-209-6262 509-290-5734	PROJECT NAME Gustin Levee	
PROJECT NUMBED DATE STARTED EXCAVATION CON	7/21/15	COMPLETED _7/21/15	PROJECT LOCATION Spokane County GROUND ELEVATION 2016 ft TEST PIT SIZE 30 inches GROUND WATER LEVELS:	
LOGGED BY PTN		CHECKED BY PTN		
EPTH (ft) PLE TYPE JMBER	GRAPHIC LOG		MATERIAL DESCRIPTION	
0.0 WEZ = 0.0 SI	M 4.0			2012.
	M 8.0	(SM) SILTY SAND, fine to (Alluvium)	medium grained, brown, moist.	2008.
		End of test pit. Groundwater not encounted	ered	
7.5		Test pit immediately backf	illed.	



TEST PIT NUMBER TP-3 PAGE 1 OF 1

CLIE	NT NALI			290-573	PROJECT NAME Gustin Levee	
		MBER 14-037B			PROJECT LOCATION Spokane County	
		ED 7/21/15		COMPLE	ETED _7/21/15 GROUND ELEVATION _2015 ft TEST PIT SIZE _30 inche	es
					ng GROUND WATER LEVELS:	
EXC	AVATION	METHOD Backhoo	е		AT TIME OF EXCAVATION Not encountered	
LOG	GED BY _	PTN	(CHECKE	ED BY PTN AT END OF EXCAVATION Not encountered	
NOT	ES				AFTER EXCAVATION Not encountered	
EVEE.GPJ O DEPTH O (ft)	SAMPLE TYPE NUMBER	TESTS	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	
GENERAL BH / TP / WELL - GINT STD US LAB.GDT - 7/23/15 15:54 - J.Y. IPEC PROJECTS/ 2014 PROJECTS/14-037B GUSTIN LEVEE FOR PAINTED HILLS/GINT/14-037B GUSTIN LEVEE.GPJ C C C C C C C C C	-	MC = 6% Fines = 22%	SM		(SM) FILL: Silty Sand with Gravel, fine to medium grained, brown, moist. (SM) SILTY SAND, fine to coarse grained, a trace of Gravel, brown, moist. (Glacial Outwash)	2009.5
LAB.GDT - 7,			•	, - , - , - ,	End of test pit.	
T STD US					Groundwater not encountered	
/ELL - GIN					Test pit immediately backfilled.	
GENERAL BH / TP / M						



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TEST PIT NUMBER TP-4 PAGE 1 OF 1

<u> </u>		1	<u>ر</u>	Telepho	ne: 509-209-62 9-290-5734		
CLIEN	IT NALE	Black		1 4%. 00			PROJECT NAME Gustin Levee
PROJ	ECT NUN	IBER	14-037	7B			PROJECT LOCATION Spokane County
DATE	STARTE	D _7/	21/15		COMPLETED	7/21/15	GROUND ELEVATION 2017 ft TEST PIT SIZE 30 inches
							GROUND WATER LEVELS:
					CHECKED BY	<u>PTN</u>	
NOTE	s	I					AFTER EXCAVATION Not encountered
O DEPTH (ft)	SAMPLE TYPE NUMBER	U.S.C.S.	GRAPHIC LOG				MATERIAL DESCRIPTION
2.5		SM	n n n n n n n n n n n n n n n n n n n	(S	SM) FILL: Sil	Ity Sand with G	ravel, fine to coarse grained, brown, moist.
J		SM		(S (C	SM) SILTY SA Glacial Outwa		/EL, fine to coarse grained, brown, moist.
			<u> 44-47-1</u> 7	7.0			2010
				_			
				Er	nd of test pit.		
				G	roundwater r	not encountered	1
				T	est nit immed	diately backfille	1
				16	or by minio	diatory backille	d.



TEST PIT NUMBER TP-5

CLU	ENT NAI	Black	ROJECT NAME Gustin Levee				
		MBER 14-037B				ROJECT LOCATION Spokane County	
		ED 7/21/15		COMPLET		ROUND ELEVATION 2021 ft TEST PIT SIZE 30 inches	
						ROUND WATER LEVELS:	
EXC	AVATION	METHOD Backho	е			AT TIME OF EXCAVATION Not encountered	
LOG	GED BY	PTN	(CHECKED	BY PTN	AT END OF EXCAVATION Not encountered	
NOT	ES					AFTER EXCAVATION Not encountered	
VEE.GPJ O DEPTH O (#)	SAL	TESTS	U.S.C.S.	GRAPHIC LOG		MATERIAL DESCRIPTION	
GENERAL BH / TP / WELL - GINT STD US LAB.GDT - 7/23/15 15:54 - J.* IPEC PROJECTS/ 2014 PROJECTS/14-037B GUSTIN LEVEE FOR PAINTED HILLS/GINT/14-037B GUSTIN LEVEE.GFU C		MC = 7% Fines = 48%	SM	5.0		D with GRAVEL, fine to coarse grained, brown, moist.	2016.0
7/23/15 15:54 - J:_IPEC	-		SM	7.0			2014.0
S LAB.GDT					End of test pit.		
IT STD U					Groundwater not	encountered	
L - GIN					Test pit immediate	ely backfilled.	
GENERAL BH / TP / WEL							



TEST PIT NUMBER TP-6

PROJI DATE EXCA' EXCA' LOGG	STARTE VATION VATION ED BY	Black MBER 14-037B D 7/21/15 CONTRACTOR A METHOD Backho		Excavating	PROJECT NAME Gustin Levee PROJECT LOCATION Spokane County GROUND ELEVATION 2021 ft TEST PIT SIZE 30 inches GROUND WATER LEVELS: AT TIME OF EXCAVATION Not encountered AFTER EXCAVATION Not encountered	
VEE.GPJ O DEPTH O (ff)	SAMPLE TYPE NUMBER	TESTS	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	
GENERAL BH / TP / WELL - GINT STD US LAB.GDT - 7/23/15 15:54 - J.: IPEC PROJECTS\ 2014 PROJECTS\ 14.037B GUSTIN LEVEE.GPJ C		Fines = 25%	SM	5.1 5.0 5.0 5.0 6.0 6.0 6.0 6.0 6.0 6.0 6.0 6	(GM) SILTY GRAVEL with SAND, fine to coarse grained, brown, moist. (Glacial Outwash)	2016.0
GENERAL BH / TP / WELL - GINT STD US LAB.GDT -					End of test pit. Groundwater not encountered Test pit immediately backfilled.	



REL	RELATIVE DENSITY OR CONSISTENCY VERSUS SPT N-VALVE									
COARSE	-GRAINED SOILS	FINE-GRAINED SOILS								
DENSITY	N(BLOWS/FT)	CONSISTENCY	N(BLOWS/FT)							
Very Loose	0 - 4	Very Soft	0 - 1							
Loose	4 - 10	Soft	2 - 3							
Medium-Dense	11 - 30	Rather Soft	4 - 5							
Medium-Dense	11 - 30	Medium	6 - 8							
Dense	31 - 50	Rather Stiff	9 - 12							
Delise	31 - 30	Stiff	13 - 16							
Vary Danca	> 50	Very Stiff	17 - 30							
Very Dense	> 30	Hard	> 30							

	USCS SOIL CLASSIFICATION										
l	MAJOR DIVISIONS			GROUP DESCRIPTIONS							
Coarse-	Gravel and	Gravel	GW	Well Graded Gravel							
Grained	Gravelly Soils	(with little or no fines)	GP	Poorly Graded Gravel							
Soils	<50% coarse fraction	Gravel	GM	Silty Gravel							
	passes #4 sieve	(with >12% fines)	GC	Clayey Gravel							
<50%	Sandy and	Sand	SW	Well Graded Sand							
passes #200	Sandy Soils	(with little or no fines)	SP	Poorly Graded Sand							
sieve	>50% coarse fraction	Sand	SM	Silty Sand							
	passes #4 sieve	(with >12% fines)	SC	Clayey Sand							
Fine-			ML	Silt							
Grained	Silt and Clay		CL	Lean Clay							
Soils	Liquid Limit < 50		OL	Organic Silt and Clay (low plasticity)							
>50%			MH	Inorganic Silt							
passes #200	Salt and Clay		CH	Fat Clay							
sieve	Liquid Limit > 50		ОН	Organic Clay and Silt (med to high plasticity)							
	Highly Organic Soils		PT	Peat Muck							

MODIFIERS								
DESCRIPTION	RANGE							
Occasional	<5%							
Trace	5% - 12%							
With	>12%							

MO	MOISTURE CONTENT							
DESCRIPTION	FIELD OBSERVATION							
Dry	Absence of moisture, dusty, dry to the touch							
Moist	Dry of optimum moisture content							
Wet	Wet of optimum moisture content							

MAJOR DIVISIONS WITH GRAIN SIZE									
SIEVE SIZE									
	12" 3" 3/4" 4 10 40 200								
GRAIN SIZE (INCHES)									
12 3 0.75 0.19 0.079 0.0171 0.0029									
Boulders	ulders Cobbles		Gravel		Sand		Silt and Clay		
Doulders	iders Cobbles	Coarse	Fine	Coarse	Medium	Fine	Silt allu Clay		

APPENDIX C LABORATORY TEST RESULTS

Inland Pacific Engineering Company 3012 North Sullivan Road, Suite C Spokane Valley, WA 99216

GRAIN SIZE DISTRIBUTION

Telephone: 509-209-6262

Fax: 509-290-5734 PROJECT NAME Gustin Levee CLIENT NAI Black PROJECT NUMBER 14-037B PROJECT LOCATION Spokane County U.S. SIEVE OPENING IN INCHES 6 4 3 2 1.5 1 3/4 U.S. SIEVE NUMBERS | 810 14 16 20 30 40 50 60 100 140 200 HYDROMETER 3 1/23/8 100 95 90 85 80 75 70 65 PERCENT FINER BY WEIGHT 7/23/15 15:31 - J.\. IPEC PROJECTS_2014 PROJECTS\14-037B GUSTIN LEVEE FOR PAINTED HILLS\GINT\14-037B GUSTIN LEVEE.GPJ 60 55 50 Ø 45 40 35 30 25 20 15 10 5 0.01 0.001 **GRAIN SIZE IN MILLIMETERS GRAVEL** SAND COBBLES SILT OR CLAY

٤1			coarse	tine	coarse	meaium	fine						
5													
1	BORE	HOLE DEP	TH		Cla	ssification			LL	PL	PI	Сс	Cu
	● B-1	5.0	0	CL-ML Sandy Silty Clay						20	4		
5.5	■ B-2	7.	5		SM	Silty Sand							
3	▲ B-2	15.0	0 S	P-SM Poorl	y Grade	d Sand with	Silt and Grave	el				1.39	12.01

<u>`</u> ★	B-3	5.0	CL-ML Sandy Silty Clay						19	4		
B.G.D.	B-3	10.0		SP-SM Poorly Graded Sand with Silt							1.48	7.85
	OREHOLE	DEPTH	D100	D60	D30	D10	%Gravel	%Sand		%Silt	%(Clay

S	ים	OILLIOLL		1 100	D00	D30	D10	70 Glavei	70 0 ana	700111	70 Clay
JTD (•	B-1	5.0	0.075						77	7.0
GINT		B-2	7.5	9.5	0.411			1.9	63.9	34	1.2
	▲ I	B-2	15.0	19	4.142	1.408	0.345	35.7	58.2	6	.1
N SIZ	*	B-3	5.0	0.075						61	1.7
GRAIN SIZE	\odot	B-3	10.0	15.9	1.234	0.537	0.157	12.8	79.4	7	.8

•

•

S

STD • **B-4**

GINT \blacksquare TP-1

B-4

B-5

B-8

TP-1

BOREHOLE

4.0

DEPTH

2.5

15.0

10.0

10.0

4.0

D100

9.5

12.5

25

25

9.5

D60

0.494

2.893

3.369

7.153

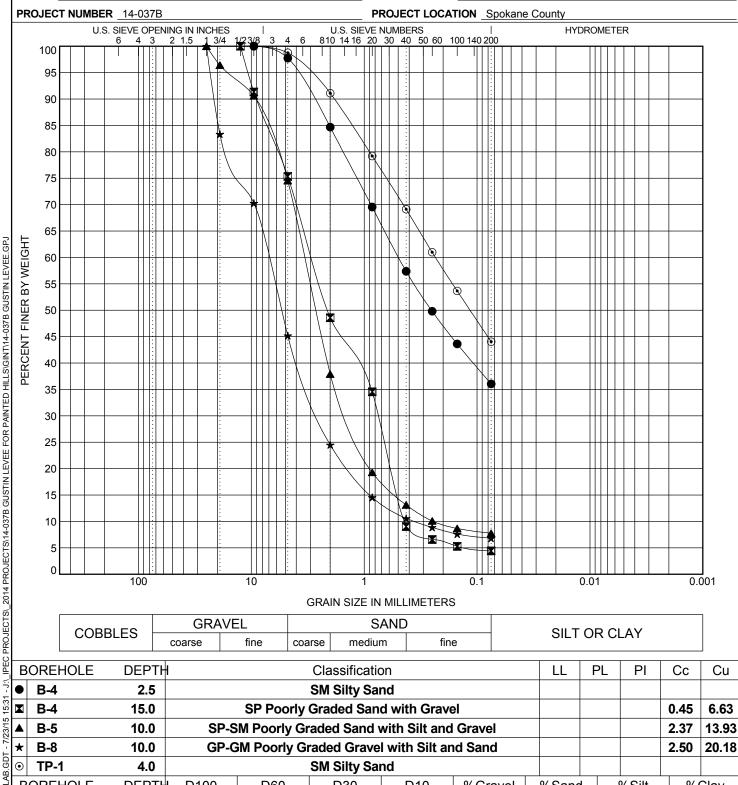
0.233

Inland Pacific Engineering Company 3012 North Sullivan Road, Suite C Spokane Valley, WA 99216 Telephone: 509-209-6262

GRAIN SIZE DISTRIBUTION

Fax: 509-290-5734

PROJECT NAME Gustin Levee **CLIENT** NAI Black



SM Silty Sand

D10

0.436

0.242

0.354

%Gravel

2.3

24.7

25.5

54.8

1.2

%Sand

61.7

70.9

66.8

38.4

54.7

%Silt

36.1

4.4

7.8

6.8

44.0

%Clay

D30

0.751

1.391

2.518

GRAIN SIZE DISTRIBUTION

PROJECT NAME Gustin Levee

